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**II CONGRESO DE VIALIDAD Y TRANSITO  
Ciudad de Encarnacion – Paraguay**

**Date 6<sup>th</sup> October 2016**

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# Polymer Modified Asphalt

## *OPTIMISED PAVEMENT DESIGN FOR BETTER ROADS*



# Use of Modified Bituminous Binders & Value Engineering

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## *Key Projects – Recent*

**Hamburg Container handling yards pavements (Germany)**

**New Doha International Airport (Qatar)**

**Dubai International Airport (UAE)**

**Muscat International Airport (Oman)**

**Abu Dhabi International Airport (UAE)**

**Suweihan Airbase (UAE)**

**Al Udaid USAF Airbase (Qatar)**

**Sohar International Airport (Oman)**

**Jeddah International Airport (Saudi Arabia)**

**Emirates Highway - 210 kms (12 lane) (UAE)**

**Sheikh Zayed Highway - 90 kms (12 lane) (UAE)**

**Bid Bid Sur Highway -120 kms (dual lane) (Oman)**

**Thumrait – Salalah Highway - 75 kms (dual lane) (Oman)**

**Qatar Express Way upgrade Program – 890 kms (varying lane provision)  
(Qatar)**



**Muscat Ring Highway – 70 kms (8 lane) (Oman)**

**Jeddah – Mecca Highway – 92 kms (6 lane) (Saudi Arabia)**

**Mafraq – Gweifat Truck Express way - 110 kms (8 lane) UAE/Saudi Arabia  
(First concessionary road project in ME)**

**Dubai Dry dock Road system (UAE)**

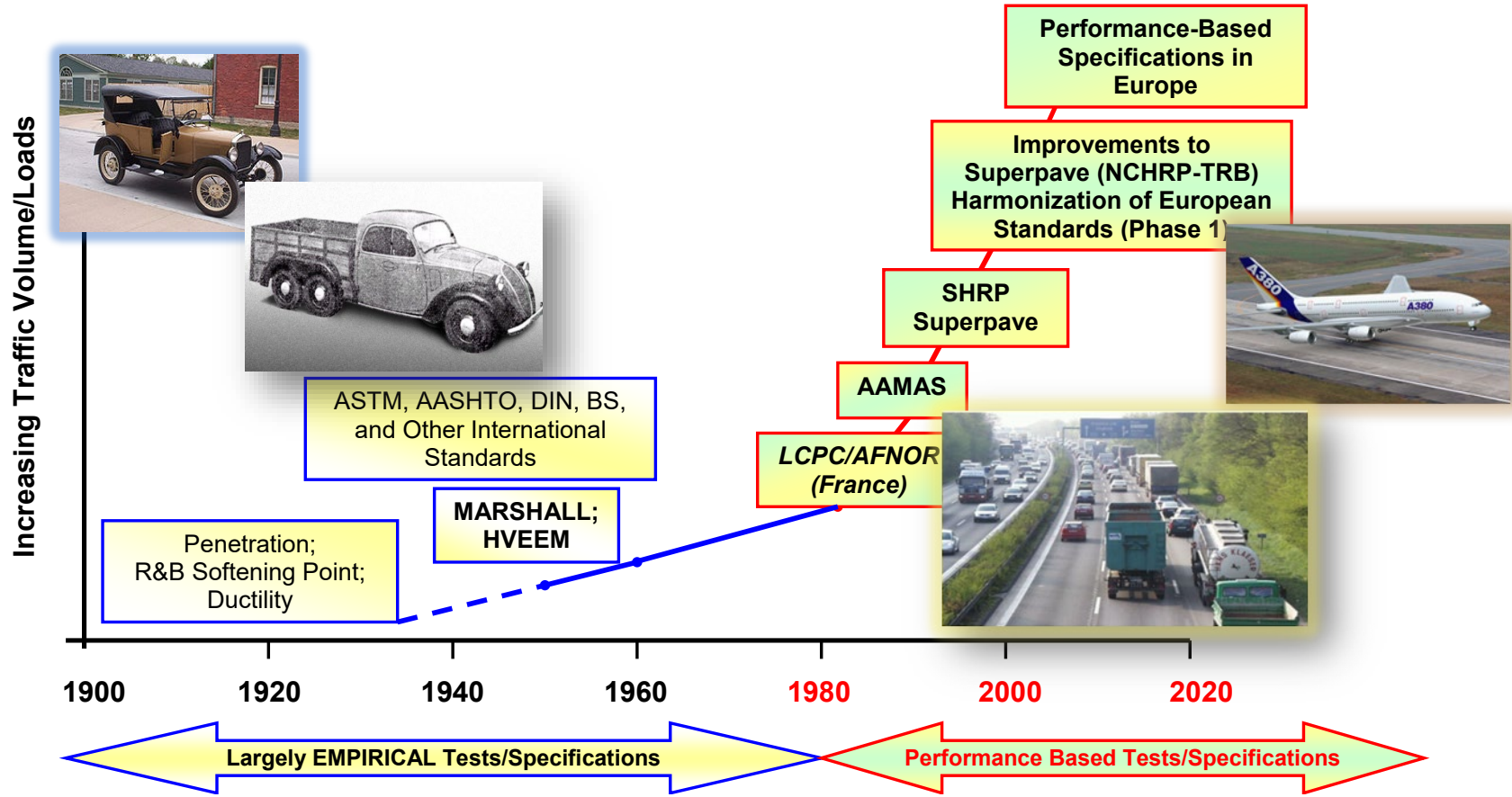
**Khalifa Port Road System and container handling area pavements (UAE)**

**Hambantota International Airport (Sri Lanka)**

**Colombo – Hambantota Airport Highway 62 kms (6 lane) (Sri Lanka)**

# ASPHALT TECHNOLOGY HISTORY

## Specifications for Bituminous Binders and Mixtures



# Rutting

- ▶ Problem identified since mid 1980s' in the US and 1970s' in Europe
- ▶ Heavy loads and high tyre pressures in trucks; stiffer tyre side walls
- ▶ Need bituminous binders with higher stiffness and adequate elasticity at high service temperatures, higher traffic densities and loads

# Rutting





# Rutting



# Rutting



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# Fatigue Cracking

- ▶ Increased truck traffic
- ▶ Repeated loads cause fatigue cracking
- ▶ Need bituminous binders with lower stiffness and higher elasticity at intermediate service temperatures (after ageing)

# Fatigue Cracking





# Fatigue Cracking



# Low Temperature Thermal Cracking

- ▶ Occurs at low service temperatures
- ▶ Unmodified bitumen cannot cope with the range of extreme minimum and maximum pavement temperatures
- ▶ Need bituminous binders with lower stiffness at low temperatures and higher stiffness at high temperatures

# Low Temperature Thermal Cracking





# Advantages of Modified Binders

- ▶ Stiffens the binder and the asphalt mix at high temperatures (minimize rutting)
- ▶ Softens the binder at low service temperatures (minimize low temperature cracking)
- ▶ Improve fatigue resistance especially where high strains are imposed on the bituminous mix



# Advantages of Modified Binders



- ▶ Improves aggregate–bitumen bonding (reduce stripping potential)
- ▶ Improve bituminous pavement durability (increase pavement service life and reduce **life–cycle costs**)
- ▶ Provide thicker binder films on aggregate in special mixes (stone matrix asphalt and open graded asphalt friction courses)

# Development and Use of Modified Binders in Europe

- ▶ Europeans ahead of the US in the 1970s'
- ▶ Modification became attractive because cost of bitumen had increased
- ▶ Highway authorities expect innovations in bituminous pavement technology from contractors who warrant the projects – concessions toll model
- ▶ Higher quality material preferred to reduce life-cycle costs

# Development and Use of Modified Binders in Europe

- ▶ Beside improving the performance of hot mix asphalt, modified binders highly successful in:
  - Durable surface dressing
  - Thin HMA wearing course
  - Open graded friction course (porous asphalt)
  - Durable slurry seals (micro surfacing)
  - Asphalt wearing course on bridge decks

# North America Experience



- ▶ Rutting problem experienced in 1980s
- ▶ No unified guide specifications for modified binders from Europe
- ▶ Proliferation of different types of modified binders created confusion
- ▶ AASHTO Task Force 31 developed different specifications for different PMBs (elastomers and plastomers) in 1992



# North America Experience

- ▶ Separate specification was available for CRMB (called Asphalt–Rubber in the US)
- ▶ Specifications based on empirical tests such as penetration, softening point and ductility similar to Europe

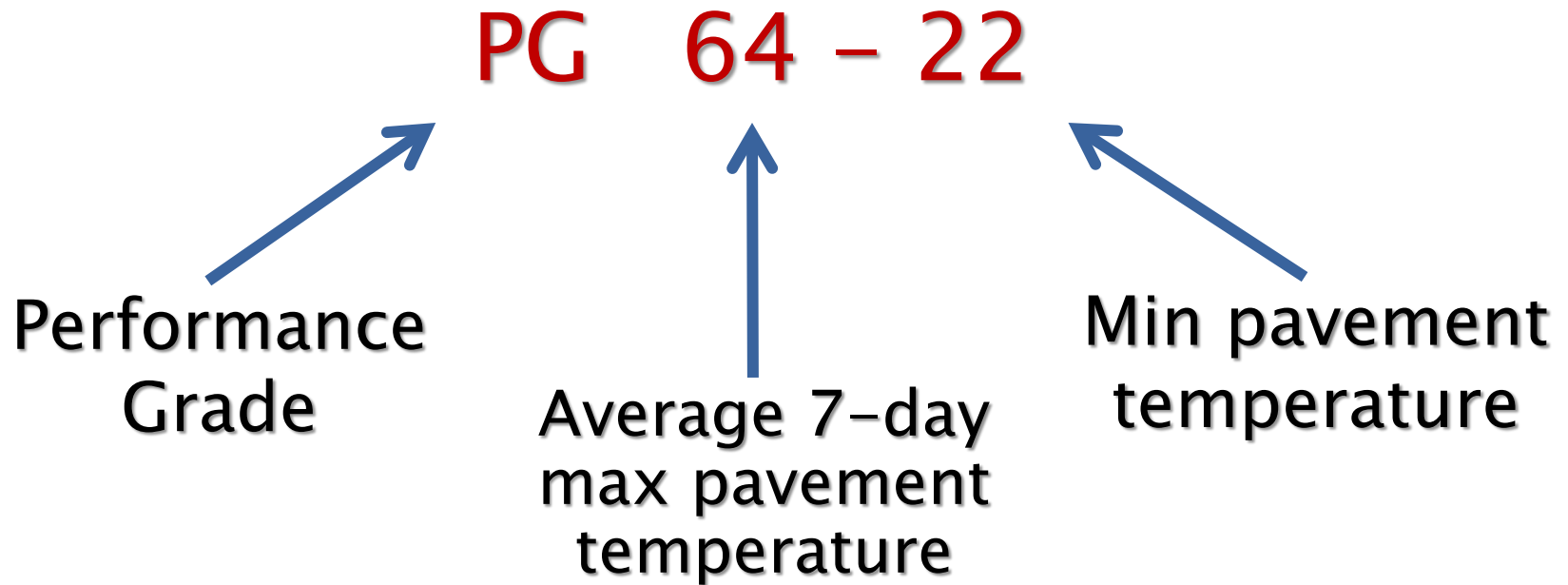
# North America Experience

- ▶ Performance based bituminous binder specification developed in SHRP Project (1987–1992) to characterize binder from low to intermediate to high service temperatures
- ▶ Dynamic Shear Rheometer (DSR) used to measure visco–elastic properties of PG binders such as  $G^*$  and phase angle delta

# Superpave Asphalt Binder Specification Grades



The grading system is based on climate



# North America Experience

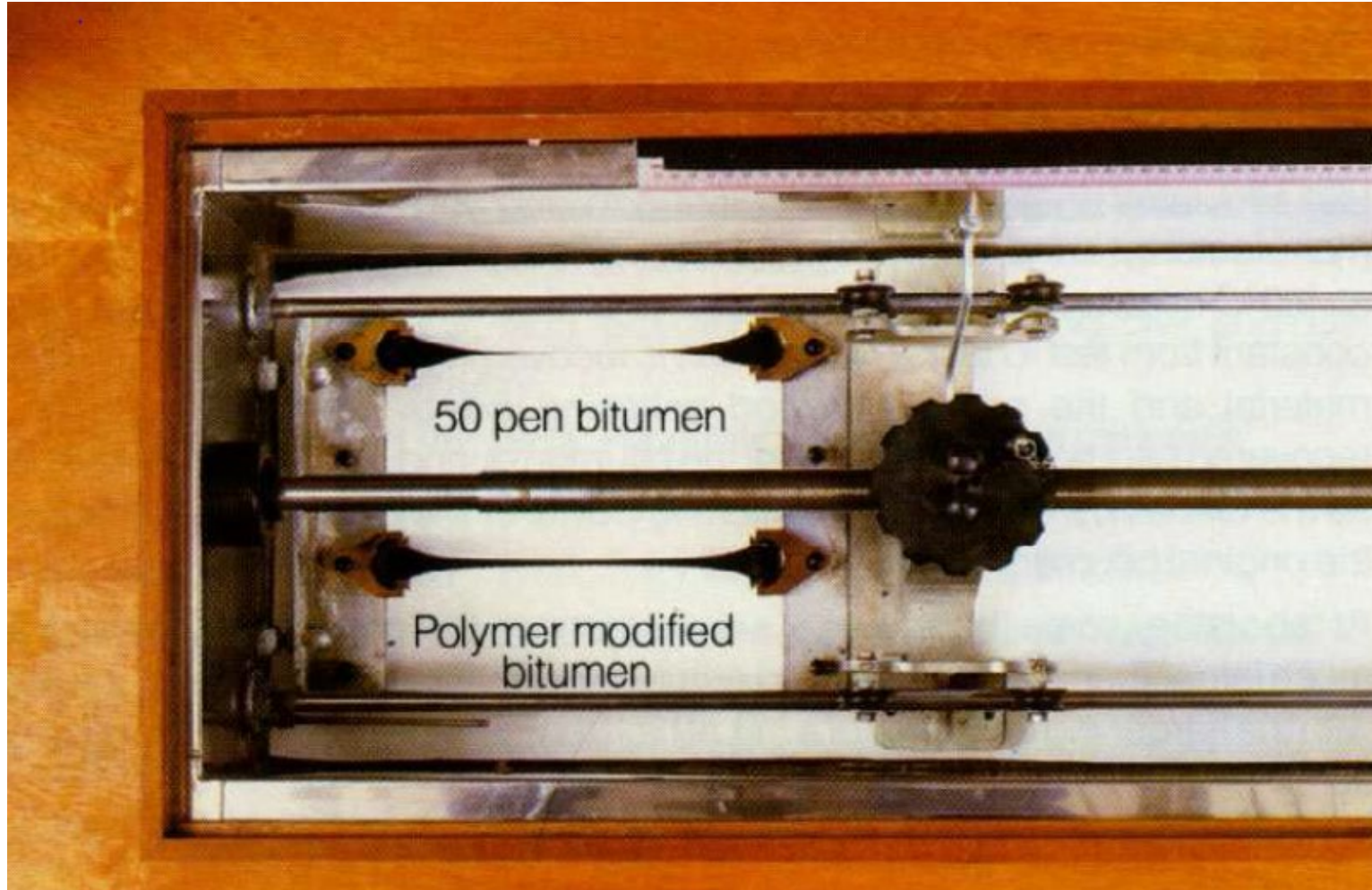
- ▶ PG Plus specifications adopted by some states to ensure adequate amount of elastomer in the PMB
- ▶ Phase angle of 75 degrees or less
- ▶ Elastic recovery using ductility machine

# Superpave Asphalt Binder





# Superpave Asphalt Binder





# Polymers

- ▶ “Polymer” simply refers to a very large molecule made by chemically reacting many (poly) smaller molecules (monomers)
- ▶ Long chains or clusters
- ▶ Sequence and chemical structure of the monomers determines the physical properties of a polymer
- ▶ Random or block copolymers are made from different types of monomers

# General Categories of Modified Binders

- ▶ Elastomers
- ▶ Plastomers



# Elastomers

- ▶ Can be stretched like a rubber band and recover the shape when the force is released
- ▶ Adds a little strength to bitumen, gets stronger when stretched (strained)
- ▶ Examples: SBS (styrene–butadiene–styrene) and ETP (ethylene tar polymer)

# Elastomers



# Plastomers

- ▶ Form a tough, rigid network within the bitumen
- ▶ Give high initial strength to bitumen to resist heavy loads
- ▶ May crack at high strains
- ▶ Examples: EVA (ethylene vinyl acetate) and polyethylene



# Compatibility of Polymers with Bitumen

- ▶ Paving bitumen has complex chemical composition, colloidal structure, physical and chemical properties
- ▶ Polymers although distinct are also complex systems
- ▶ **Bitumen–Polymer relationship is very complex**

# Polymer mixed with Hot Bitumen (Optimisation Stage I)

- ▶ Mix is heterogeneous because polymer and bitumen are not **compatible** initially
- ▶ Both tend to separate from each other
- ▶ Initial mix does not behave like a typical bitumen

# Polymer mixed with Hot Bitumen (Optimisation Stage II)

- ▶ Mix is totally homogeneous even at molecular level because polymer and bitumen are compatible
- ▶ Resulting binder is stable
- ▶ Improvement in service quality of binder is slight, only its viscosity increases



# **Polymer mixed with Hot Bitumen (Optimisation Stage III)**

- ▶ Mix is micro heterogeneous and consists of two distinct finely interlocked phases
- ▶ Compatible polymer absorbs some of the oily fractions of bitumen and swells to form a polymer phase distinct from residual bitumen
- ▶ Resulting binder has genuinely modified properties

# Crumb Rubber Modified Bitumen (CRMB)

- ▶ When highway engineers in the US were trying to understand complex PMBs
- ▶ Came another far more complex and least understood modified binder: CRMB
- ▶ Rubber from discarded tyres is ground to crumb and then added to bitumen
- ▶ Called Asphalt–Rubber (AR) in the US



# Crumb Rubber Modified Bitumen (CRMB)



# Properties of CRMB

- ▶ Properties of CRMB depend on:
  - Bitumen crude **source** and **method** of refining
  - **Source** of crumb rubber (truck/car tyres; tread; sidewalls)
  - Grinding of rubber (**ambient or cryogenically cooled**)
  - Amount and **size** of crumb rubber

# Use of CRMB in the US

- ▶ Use of CRMB not significant until 1991
- ▶ **Mixed** pavement **performance results**
- ▶ US Congress mandated its use in all 50 states in 1991 (strong rubber/environmental lobby)
- ▶ US FHWA Training Program in Qc/QA of CRMB in 1992
- ▶ Mandate ended in 1995

# CRMB Quality Control Requirements

- ▶ Crumb rubber tends to **separate** and **settle down** in bitumen (**needs mechanical agitation to keep in suspension**)
- ▶ Crumb rubber prone to **degradation** (de-vulcanization / de-polymerization) when kept hot for extended period of time (**must be used within 6-8 hours**)

# Value Engineering Design Concept

- ▶ Highly modified HiMA mixes (7.5% SBS) may be used in construction of pavements with lower layer thicknesses than those using standard modified mixes (3.0% SBS)
- ▶ Without a decrease in the resistance to fatigue and rutting and other performance characteristics



# Value Engineering Example

Type of Layer	Type of Material	Standard SBS Thickness (cm)	HiMA Thickness (cm)
Asphalt pavement layer	Asphalt Concrete Standard SBS: PG 76S-22 HiMA: PG 76E-22	12.0	7.0
Cement stabilized based	Stabilized with Cement Portland	30.0	25.0
Non-stabilized layer	A-2-4	30.0	30.0
Soil	A-4	Semi-infinite	Semi-infinite



# Value Engineering Design Approach

- ▶ Use Darwin–ME for pavement design
  - Allows use of laboratory to determine damage model coefficients (Fatigue and rutting)
  - Performance Calibrated
  - Quickly evaluate effect of thickness reduction on fatigue and rutting performance
  
- ▶ Determine fatigue and rutting coefficients in laboratory with std. lab tests
  - For both – HiMA and standard mixes

# Value Engineering Design

## Assumptions

- ▶ Darwin–ME may be used to predict performance of standard and HiMA mixes using laboratory determined coefficients
  - K factors (global) rather than  $\beta$  factors (local)
- ▶ Predicted performance is **conservative**
- ▶ Performance may be further calibrated using field performance data
  - $\beta$  factors

# What is HiMA?

- ▶ HiMA technology uses modified binders with high content of SBS polymer (>7%) to produce hot mix asphalt.
- ▶ High polymer content gives a phase inversion, so the binder acts more like asphalt–modified rubber than rubber–modified asphalt with much higher toughness and resilience
- ▶ Increased toughness allows for the construction of pavements with **lower thickness** than traditionally modified binders **without a decrease in the resistance to fatigue and deformation.**

# Value Engineering Design Approach

- ▶ Determine optimum layer thickness of the wearing course and base layers for high modulus mixes made using HiMA
  - Use Darwin–ME for pavement design method
    - Mechanistic–empirical design method
    - Requires mix modulus master curve
    - Requires fatigue model coefficients –  $k_f$  factors for HiMA
    - Requires rutting model coefficients –  $k_r$  factors for HiMA
  
- ▶ Compare HiMA layer thicknesses to those determined for standard SBS
  - Determine cost savings for a given life expectancy

# Darwin-ME Data Needs...

- ▶ Pavement Design Data for Darwin-ME
- ▶ Materials Data
  - Dynamic Modulus of mixes – mix design volumetrics etc.
  - Binder  $G^*$  and phase angle
  - Fatigue coefficients – kf factors
    - endurance limit
  - Rutting coefficients – kr factors
- ▶ Unbound layers and subgrade soil data
- ▶ Traffic data
- ▶ Climate data
- ▶ Performance criteria



# Mix and Binder Data

- ▶ **Dynamic Modulus – AMPT Testing on Mixes**
  - Test Temperatures: 10, 40, 68, 100 and 130°F
  - Test Frequencies: 0.1, 1.0, 5.0 and 10Hz
- ▶ **Binder  $G^*$  and phase angle, same temps at 10 rad/s**
  - DSR testing on Binders
- ▶ **Fatigue data at one temperature 20C and 4 strain levels**
  - AMPT Pull–Pull test S–VECD approach (Dr. R. Kim)
- ▶ **Rutting data**
  - AMPT Flow number data at 3 temperatures
  - NCHRP 9–30A protocol – Mr. Harold von Quintus

# Conventional SBS Modified Asphalt PG 76-22

- ▶ Tests completed in small samples containing typical Asphalt binder, Penetration Grade 85/100 and PG: PG 58-28, Pen 60/70
- ▶ Asphalt binder modified with standard SBS polymer (3.5%)
- ▶ Properties and PG values:
  - Softening point: 60°C
  - Elastic recovery: 5% at 25°C after RTFO
  - Conventional or standard PG: PG 76-22
  - PG with MSCR PG: PG 76-22S ( $U_{nr} > 4 \text{kPa}^{-1}$ )

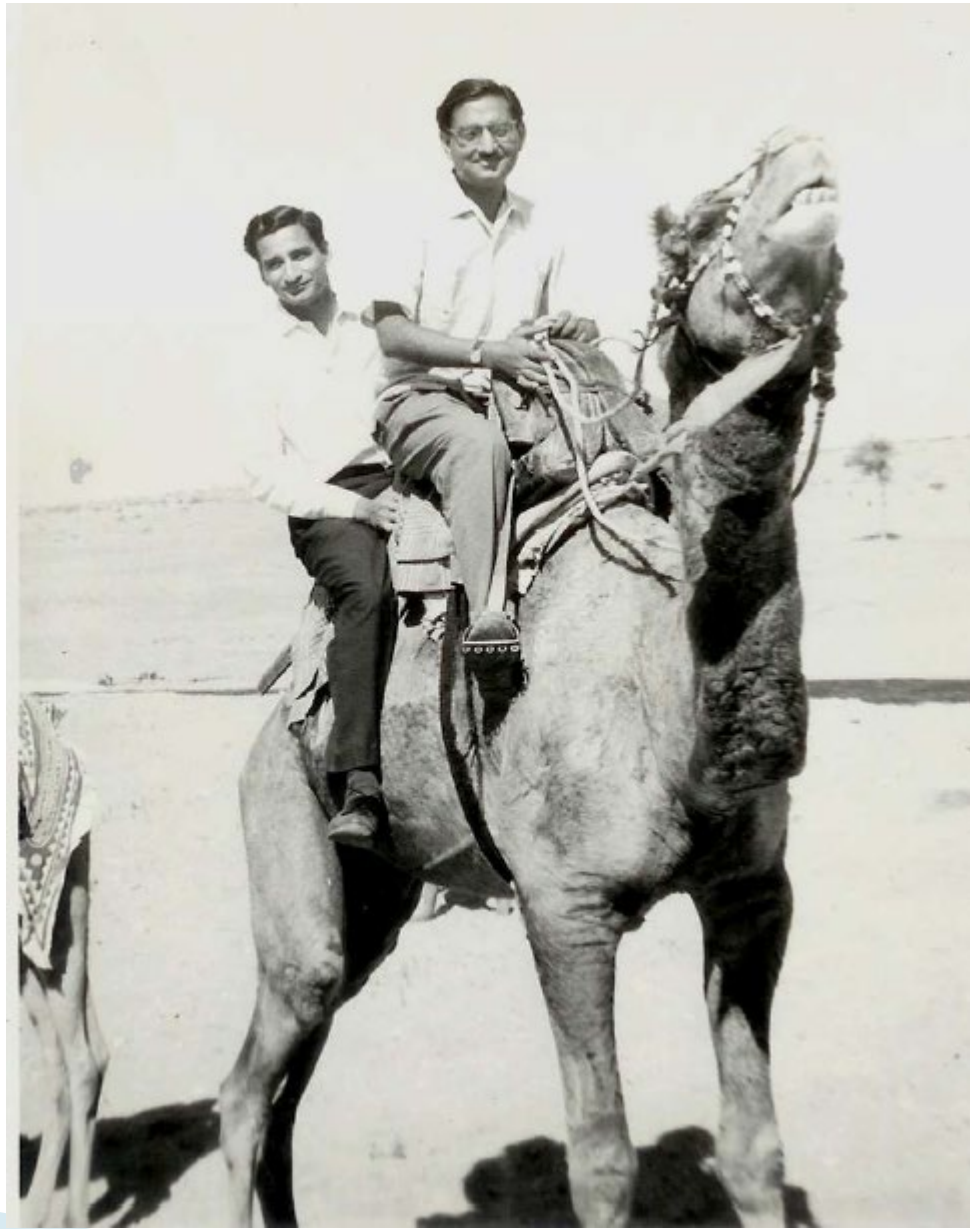
# Highly Modified Asphalt Binder (HiMA) PG76E-22



- ▶ Asphalt binder: Typical Refinery
  - Penetration grades: 85/100, PG 58-28 (PG 62.33-29.88) and PEN 60/70
- ▶ Polymer type: Reactive radial SBS
  - Polymer content: 7.5%
- ▶ HiMA – Highly Modified PMA
  - Softening point, SP: 85°C
  - Elastic recovery: 95% at 25°C after the RTFO
  - Conventional/Standard PG: PG 94-22
  - PG with MSCR: PG76-22E ( $J_{nr} = 0.1 \text{ kPa}^{-1}$ )
- ▶ The HiMA binder met the temperature and loading requirements of the project.

# Summary Findings

- ▶ Highly Modified Binders maybe used to reduce Layer Thickness
- ▶ Darwin–ME calibration factors maybe determined successfully in the Laboratory
  - AMPT Pull–Pull test and S/VECD for Fatigue Coefficients
  - NCHRP 9–30A for Rutting Coefficients



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**Muchas Gracias!**  
**Espero verlos muy pronto para un**  
**enfoque mas tecnico!**